

第九讲 梁单元

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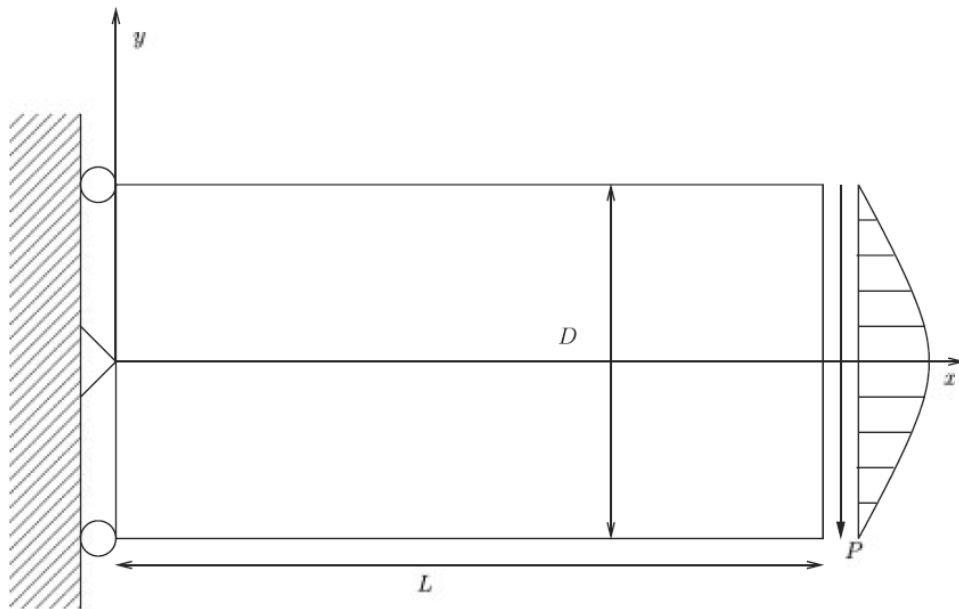
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同济大学土木工程学院



期末大作业

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二维梁的信息如下： $L=8$ ， $D=4$ ，厚度 $t=1$ ，弹性模量 $E=3.e7$ ，泊松比 $\nu=0.2$ ，荷载 $P=200$ 。试完成以下计算：

1. 自行编制二维有限元程序，采用平面四节点单元，同时考虑全积分和减缩积分，模拟梁受力之后的位移和应力。
2. 针对不同数值积分方式所得计算结果，变化合适的高宽比（1:2~1:10），讨论不同数值积分方式可能引起的锁闭、沙漏等问题。
3. 在双对数坐标中画出采用减缩积分单元所得计算所得位移、应力结果的误差范数曲线。



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COLLEGE OF CIVIL ENGINEERING

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提示：

1. 梁的位移和内力的解析解如下：

$$u_x = \frac{Py}{6EI} \left[(6L - 3x)x + (2 + \nu) \left(y^2 - \frac{D^2}{4} \right) \right]$$

$$u_y = -\frac{P}{6EI} \left[3\nu y^2 (L - x) + (4 + 5\nu) \frac{D^2}{4} x + (3L - x)x^2 \right]$$

$$\sigma_{xx} = \frac{P(L - x)y}{I}$$

$$\sigma_{yy} = 0$$

$$\tau_{xy} = \frac{P}{2I} \left(y^2 - \frac{D^2}{4} \right)$$

$$I = \frac{tD^3}{12}$$

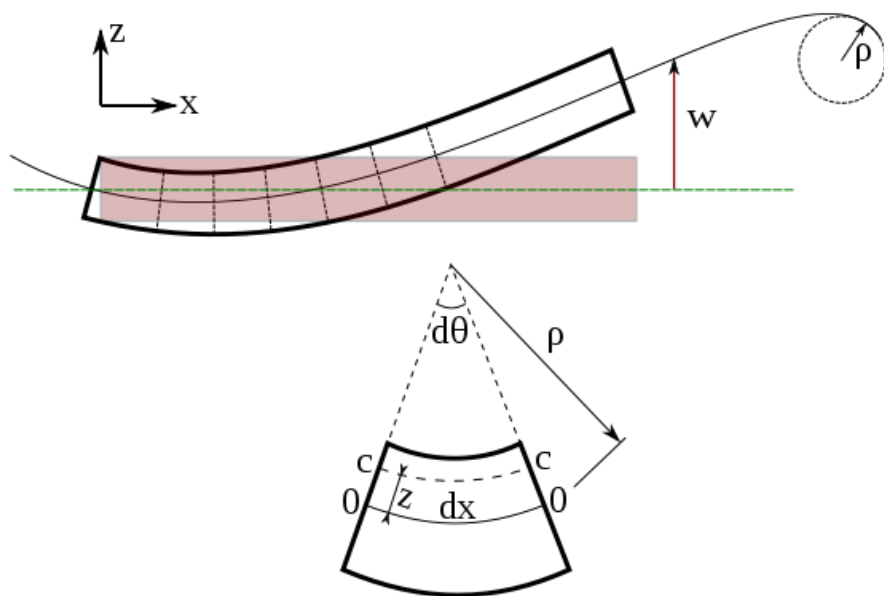
2. 误差范数的计算公式如下，计算中须至少变化5个单元长度，最大单元长度与最小单元长度之间跨过3个长度量级，采用比有限元计算过程中更加精细的数值积分，才可能得到稳定且成一条直线结果。

$$\|f - \hat{f}\|_2 = \left[\int_{\Omega} (f - \hat{f})^2 d\Omega \right]^{\frac{1}{2}}$$

3. 须根据解析解精确施加边界条件，不能随意采用圣维南原理，否则误差范数计算将会出现问题。



工程梁单元



$$\varepsilon_y = 0, \varepsilon_{xy} = 0$$

$$\tilde{u} = u - z\theta \quad \theta = \frac{\partial w}{\partial x}$$

$$\varepsilon_x = \frac{\partial \tilde{u}}{\partial x} = \frac{\partial u}{\partial x} - z \frac{\partial^2 w}{\partial x^2}$$

$$\sigma_x = E\varepsilon_x = E \left(\frac{\partial u}{\partial x} - z \frac{\partial^2 w}{\partial x^2} \right)$$

Euler-Bernoulli梁理论



工程梁单元

$$\sigma_x = E\varepsilon_x = E\left(\frac{\partial u}{\partial x} - z \frac{\partial^2 w}{\partial x^2}\right)$$

$$N = \int_A \sigma_x dA = EA \frac{\partial u}{\partial x} \quad \begin{array}{c} \frac{\partial N}{\partial x} = f(x) \\ \longrightarrow \end{array} \quad EA \frac{\partial^2 u}{\partial x^2} = f(x)$$

$$M = \int_A \sigma_x z dA = -EI \frac{\partial^2 w}{\partial x^2} \quad \begin{array}{c} \frac{\partial M}{\partial x} = Q \\ \longrightarrow \\ \frac{\partial Q}{\partial x} = -q(x) \end{array} \quad \frac{\partial^2}{\partial x^2} \left(EI \frac{\partial^2 w}{\partial x^2} \right) = q(x)$$

轴向变形与横向变形互不影响，可分别构造求解方法。



工程梁单元

轴向变形控制方程/泛函：

$$EA \frac{\partial^2 u}{\partial x^2} = f(x) \quad \Pi = \int_L \frac{EA}{2} \left(\frac{du}{dx} \right)^2 dx - \int_L f(x) u dx - \sum_j P_j u_j$$

弱形式控制方程：

$$\int_L \frac{d\delta u}{dx} EA \frac{du}{dx} dx - \int_L f(x) \delta u dx - \sum_j P_j \delta u_j = 0$$

按照单元积分：

$$\mathbf{A} \int_{L^e} \frac{d\delta u}{dx} EA \frac{du}{dx} dx - \mathbf{A} \int_{L^e} f(x) \delta u dx - \sum_j F_j \delta u_j = 0$$



工程梁单元

插值: $u = \sum_{i=1}^n N_i(\xi) u_i = \mathbf{N} \mathbf{u}^e$

两节点单元: $N_1(\xi) = 1 - \xi \quad N_2(\xi) = \xi \quad 0 \leq \xi \leq 1$

$$\mathbf{K} \mathbf{d} = \mathbf{P}$$

$$\mathbf{K} = \sum_{e=1}^{n_{el}} \mathbf{K}^e, \mathbf{K}^e = \int_0^1 \frac{EA}{l} \left(\frac{d\mathbf{N}}{d\xi} \right)^T \frac{d\mathbf{N}}{d\xi} d\xi$$

$$\mathbf{K}^e = \frac{EA}{l} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$\mathbf{P} = \sum_{e=1}^{n_{el}} \mathbf{P}^e, \mathbf{P}^e = \int_0^1 (\mathbf{N})^T f(\xi) l d\xi + \sum_j \mathbf{N}^T(\xi_j) P_j$$



工程梁单元

侧向变形控制方程/泛函：
$$\frac{\partial^2}{\partial x^2} \left(EI \frac{\partial^2 w}{\partial x^2} \right) = q(x)$$

$$\Pi = \int_L \frac{EI}{2} \left(\frac{d^2 w}{dx^2} \right)^2 dx - \int_L q(x) w dx - \sum_j P_j w_j + \sum_k M_k \theta_k$$

弱形式控制方程：

$$\int_L \frac{d^2 \delta w}{dx^2} EI \frac{d^2 w}{dx^2} dx - \int_L q(x) \delta w dx - \sum_j P_j \delta w_j + \sum_k M_k \delta \theta_k = 0$$

按照单元积分：

$$\mathbf{A} \int_{L^e} \frac{d^2 \delta w}{dx^2} EI \frac{d^2 w}{dx^2} dx - \mathbf{A} \int_{L^e} q(x) \delta w dx - \sum_j P_j \delta w_j + \sum_k M_k \delta \theta_k = 0$$



工程梁单元

$$w(\xi) = \sum_{i=1}^2 H_i^{(0)}(\xi) w_i + \sum_{i=1}^2 H_i^{(1)}(\xi) \theta_i = \mathbf{N} \mathbf{a}^e \quad \mathbf{a}^e = [w_1, \theta_1, w_2, \theta_2]$$

$$N_3 = 1 - 3\xi^2 + 2\xi^3, N_4 = (\xi - 2\xi^2 + \xi^3)l, N_5 = 3\xi^2 - 2\xi^3, N_6 = (-\xi^2 + \xi^3)l$$

$$0 \leq \xi \leq 1$$

$$\mathbf{K} \mathbf{a} = \mathbf{P}$$

$$\mathbf{K} = \sum_{e=1}^{n_{el}} \mathbf{A} \mathbf{K}^e, \mathbf{P} = \sum_{e=1}^{n_{el}} \mathbf{A} \mathbf{P}^e$$

$$\mathbf{K}^e = \int_0^1 \frac{EI}{l^3} \left(\frac{d^2 \mathbf{N}}{d\xi^2} \right)^T \frac{d^2 \mathbf{N}}{d\xi^2} d\xi$$

$$\mathbf{K}^e = \frac{EI}{l^3} \begin{bmatrix} 12 & 6l & -12 & 6l \\ 6l & 4l^2 & -6l & 2l^2 \\ -12 & -6l & 12 & -6l \\ 6l & 2l^2 & -6l & 4l^2 \end{bmatrix}$$

$$\mathbf{P}^e = \int_0^1 (\mathbf{N})^T q(\xi) l d\xi + \sum_j \mathbf{N}^T(\xi_j) P_j - \sum_k M_k \frac{d\mathbf{N}^T(\xi_k)}{l d\xi}$$



工程梁单元

组合杆单元与梁单元

$$\mathbf{a}_i = [u_i, w_i, \theta_i]^T \quad \mathbf{F}_i = [N_i, P_i, M_i]^T$$

插值矩阵

$$\begin{Bmatrix} u \\ v \\ \theta \end{Bmatrix} = \mathbf{N} \mathbf{a}^e = \begin{bmatrix} N_1 & 0 & 0 & N_2 & 0 & 0 \\ 0 & N_3 & N_4 & 0 & N_5 & N_6 \\ 0 & 0 & N_1 & 0 & 0 & N_2 \end{bmatrix} \begin{bmatrix} u_1 \\ w_1 \\ \theta_1 \\ u_2 \\ w_2 \\ \theta_2 \end{bmatrix}$$

单元刚度矩阵
(两端刚接)

$$\mathbf{K}^e = \begin{bmatrix} EA/l & 0 & 0 & -EA/l & 0 & 0 \\ & 12EI/l^3 & 6EI/l^2 & 0 & -12EI/l^3 & 6EI/l^2 \\ & & 4EI/l & 0 & -6EI/l^2 & 2EI/l \\ & \text{对} & & EA/l & 0 & 0 \\ & & \text{称} & & 12EI/l^3 & -6EI/l^2 \\ & & & & & 4EI/l \end{bmatrix}$$



工程梁单元

单元刚度矩阵

(一端刚接一端铰接)

$$K^e = \begin{bmatrix} EA/l & 0 & 0 & -EA/l & 0 & 0 \\ & 3EI/l^3 & 3EI/l^2 & 0 & -3EI/l^3 & 0 \\ & & 3EI/l & 0 & -3EI/l^2 & 0 \\ & \text{对} & & EA/l & 0 & 0 \\ & & \text{称} & & 3EI/l^3 & 0 \\ & & & & & 0 \end{bmatrix}$$

单元刚度矩阵

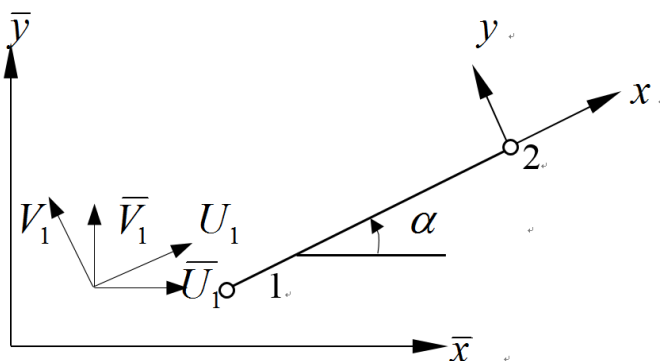
(两端铰接)

$$K^e = \begin{bmatrix} EA/l & 0 & 0 & -EA/l & 0 & 0 \\ & 0 & 0 & 0 & 0 & 0 \\ & & 0 & 0 & 0 & 0 \\ & \text{对} & & EA/l & 0 & 0 \\ & & \text{称} & & 0 & 0 \\ & & & & & 0 \end{bmatrix}$$



工程梁单元

坐标转换



$$\mathbf{F}^e = \mathbf{T} \bar{\mathbf{F}}^e$$

$$\mathbf{a}^e = \mathbf{T} \bar{\mathbf{a}}^e$$

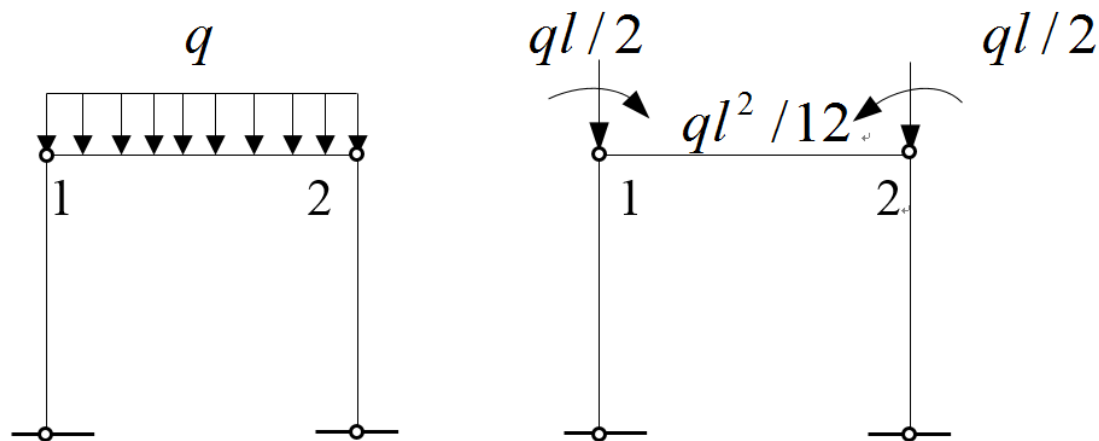
$$\bar{\mathbf{K}}^e = \mathbf{T}^T \mathbf{K}^e \mathbf{T}$$

$$\mathbf{T} = \begin{bmatrix} \cos \alpha & \sin \alpha & 0 & 0 & 0 & 0 \\ -\sin \alpha & \cos \alpha & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & \cos \alpha & \sin \alpha & 0 \\ 0 & 0 & 0 & -\sin \alpha & \cos \alpha & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$



工程梁单元

等效节点荷载

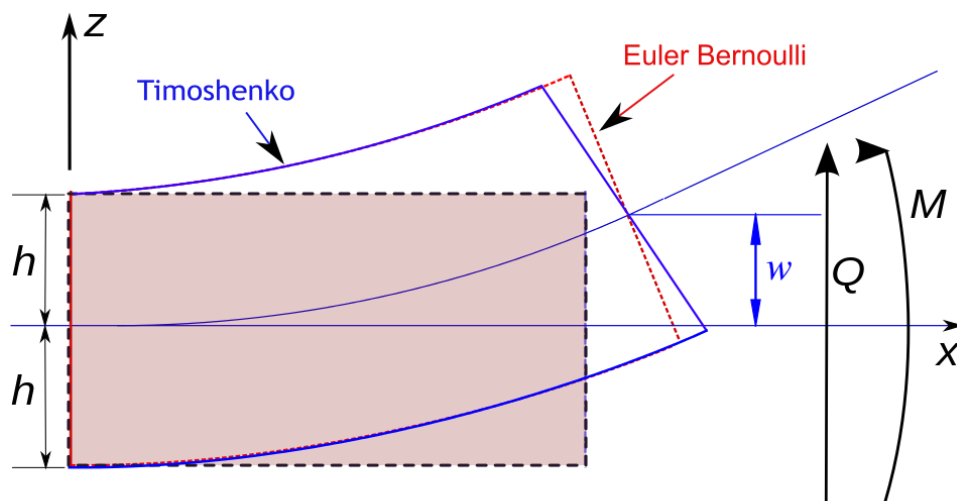


$$\mathbf{P}^e = \int_0^1 (\mathbf{N})^T q(\xi) l d\xi$$

支座约束与支座位移：罚函数法！



剪切梁单元



Timoshenko梁理论



Timoshenko medal



剪切梁单元

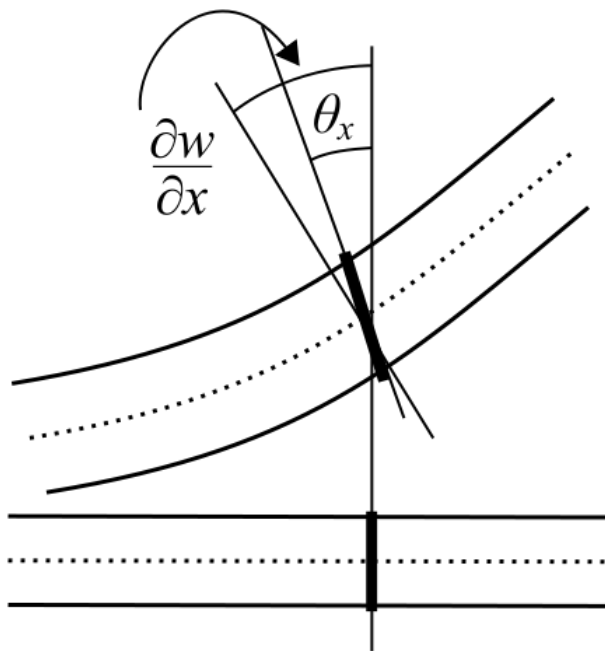
$$\gamma = \frac{dw}{dx} - \theta$$

先不考虑轴向变形：

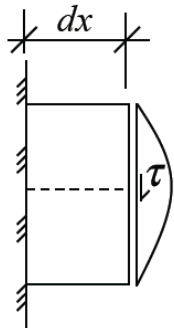
$$u = -z\theta(x), v = 0, w = w(x)$$

$$M = -EI \frac{d\theta}{dx}$$

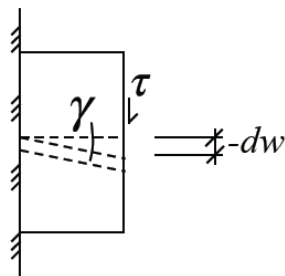
$$\frac{d^2}{dx^2} \left(EI \frac{d\theta}{dx} \right) = q(x)$$



剪切梁单元



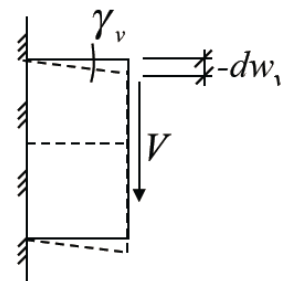
(a)
Shear stress



(b)
Fibre
deformation



(c)
Actual shear
deformation



(d)
Average shear
deformation

$$\gamma V = \int_A \tilde{\gamma} \tau dA = \int_A \frac{\tau^2}{G} dA = \int_A \frac{1}{G} \left(\frac{VS^*}{It} \right)^2 dA$$

定义: $\gamma = \frac{V}{k_s AG}$ $k_s = \frac{I^2}{A \int_A (S^*/t)^2 dA}$



剪切梁单元

$$\frac{dw}{dx} - \theta = \frac{1}{k_s AG} \frac{dM}{dx} = \frac{1}{k_s AG} \frac{d}{dx} \left(EI \frac{d\theta}{dx} \right) \quad \frac{d^2}{dx^2} \left(EI \frac{d\theta}{dx} \right) = q(x)$$

$$\frac{d^2 w}{dx^2} - \frac{d\theta}{dx} = \frac{1}{k_s AG} \frac{d^2}{dx^2} \left(EI \frac{d\theta}{dx} \right) = \frac{q(x)}{k_s AG}$$

$$\frac{d^2}{dx^2} \left[EI \left(\frac{d^2 w}{dx^2} + \frac{q(x)}{k_s AG} \right) \right] = q(x)$$

$$\frac{d^2}{dx^2} \left(EI \frac{d^2 w}{dx^2} \right) = q(x) - \frac{EI}{k_s AG} \frac{d^2 q(x)}{dx^2}$$



剪切梁单元

能量泛函：

$$\Pi = \int_L \frac{EI}{2} \left(\frac{d\theta}{dx} \right)^2 dx + \int_L \frac{Gk_s A}{2} \gamma^2 dx - \int_L q(x) w dx - \sum_j P_j w_j + \sum_k M_k \theta_k$$

弱形式：

$$\int_L \frac{d\delta\theta}{dx} EI \frac{d\theta}{dx} dx + \int_L \left(\frac{d\delta w}{dx} - \delta\theta \right) Gk_s A \left(\frac{dw}{dx} - \theta \right) dx - \int_L q(x) \delta w dx - \sum_j P_j \delta w_j + \sum_k M_k \delta\theta_k = 0$$

单元积分：

$$\mathbf{A}_{e=1}^{n_{el}} \int_{L^e} \frac{d\delta\theta}{dx} EI \frac{d\theta}{dx} dx + \mathbf{A}_{e=1}^{n_{el}} \int_{L^e} \left(\frac{d\delta w}{dx} - \delta\theta \right) Gk_s A \left(\frac{dw}{dx} - \theta \right) dx - \mathbf{A}_{e=1}^{n_{el}} \int_{L^e} q(x) \delta w dx - \sum_j P_j \delta w_j + \sum_k M_k \delta\theta_k = 0$$



剪切梁单元

位移、转角分别插值

(C0连续性, Timoshenko梁单元) :

$$w = \sum_{i=1}^n N_i w_i \quad \theta = \sum_{j=1}^n N_j \theta_j$$

$$\mathbf{K} \mathbf{a} = \mathbf{P} \quad \mathbf{K} = \sum_{e=1}^{n_{el}} \mathbf{A}^e \mathbf{K}^e, \mathbf{P} = \sum_{e=1}^{n_{el}} \mathbf{A}^e \mathbf{P}^e$$

$$\mathbf{K}^e = \int_0^1 \mathbf{B}_b^T E I \mathbf{B}_b l d\xi + \int_0^1 \mathbf{B}_s^T G k_s A \mathbf{B}_s l d\xi$$

$$\mathbf{B}_b^T = \begin{bmatrix} 0 & 0 & \dots & 0 \\ \frac{dN_1}{dx} & \frac{dN_2}{dx} & \dots & \frac{dN_n}{dx} \end{bmatrix}^T \quad \mathbf{B}_s^T = \begin{bmatrix} \frac{dN_1}{dx} & \frac{dN_2}{dx} & \dots & \frac{dN_n}{dx} \\ -N_1 & -N_2 & \dots & -N_n \end{bmatrix}^T$$

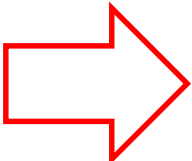
$$\mathbf{P}^e = \int_0^1 (\mathbf{N})^T \begin{bmatrix} q(\xi) \\ 0 \end{bmatrix} l d\xi + \sum_j \mathbf{N}^T(\xi_j) \begin{bmatrix} P_j \\ 0 \end{bmatrix} - \sum_k \mathbf{N}^T(\xi_k) \begin{bmatrix} 0 \\ M_k \end{bmatrix}$$

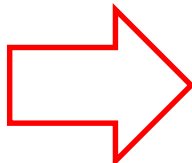


剪切梁单元

能量泛函: $\Pi = \int_L \frac{EI}{2} \left(\frac{d\theta}{dx} \right)^2 dx + \int_L \frac{Gk_s A}{2} \left(\frac{dw}{dx} - \theta \right)^2 dx$

两节点一阶线性插值: $w = \sum_{i=1}^2 N_i w_i \quad \theta = \sum_{i=1}^2 N_i \theta_i$

 $\gamma = \frac{dw}{dx} - \theta = \frac{1}{l}(w_2 - w_1) - \frac{1}{2}(\theta_1 + \theta_2) - \frac{1}{2}(\theta_1 - \theta_2)\xi$

对于浅梁, 系数成为
被动罚数! $\gamma \rightarrow 0$  $\frac{1}{l}(w_2 - w_1) = \frac{1}{2}(\theta_1 + \theta_2) \quad \theta_1 - \theta_2 = 0$ 锁死!

减缩积分: 1点高斯积分, 只在 $\xi=0$ 处取值



剪切梁单元

一般插值:

$$w = \sum_{i=1}^{n_w} N_i w_i \quad \theta = \sum_{i=1}^{n_\theta} N_i \theta_i$$

剪切应变:

$$\gamma = \frac{dw}{dx} - \theta = \sum_{i=1}^{n_w} \frac{dN_i}{dx} w_i - \sum_{i=1}^{n_\theta} N_i \theta_i$$

抛弃 $\gamma \rightarrow 0$ 可能引起锁死的项

直接代入弱形式计算:

$$\tilde{\gamma} = \sum_{i=1}^{\bar{n}_w} \tilde{B}_i w_i - \sum_{i=1}^{\bar{n}_\theta} \tilde{N}_i \theta_i \approx \gamma$$

$$\mathbf{A} \int_{L^e} \frac{d\delta\theta}{dx} EI \frac{d\theta}{dx} dx + \mathbf{A} \int_{L^e} \delta\tilde{\gamma} Gk_s A \tilde{\gamma} dx - \mathbf{A} \int_{L^e} q(x) \delta w dx - \sum_j P_j \delta w_j + \sum_k M_k \delta \theta_k = 0$$

假设剪切应变方法!



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今天就到这里，
明天的事儿明天再说！

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